Demonstrating demand response from water distribution system through pump scheduling

Ruben Menke^{a,*}, Edo Abraham^a, Panos Parpas^b, Ivan Stoianov^a

^aDepartment of Civil and Environmental Engineering, Imperial College London, SW7 2BU London, UK. ^bDepartment of Computing, Imperial College London, SW7 2BU London, UK.

Abstract

Significant changes in the power generation mix are posing new challenges and opportunities for balancing systems of the grid utilising Demand Response (DR). We explore the opportunities for a water distribution system (WDS) to provide balancing services with demand response through pump scheduling and evaluate the associated benefits. Using a benchmark network and demand response mechanisms available in the UK, these benefits are assessed in terms of reduced green house gas emissions from the grid due to the displacement of more polluting power sources and additional revenues for water utilities. The optimal pump scheduling problem is formulated as a mixed-integer optimization problem and solved using a branch and bound algorithm. The formulation finds the optimal level of power capacity to commit to the provision of demand response for a range of demand response schemes offered in the UK. We show that DR from WDS can offer financial benefits to WDS operators while providing response energy to the grid with less greenhouse gas emissions than competing reserve energy technologies. Using a Monte Carlo simulation based on data from 2014, we show that the cost of providing the storage energy is less than the financial compensation available for the equivalent energy supply. The GHG emissions from the demand response provision from a WDS are also shown to be smaller than those of contemporary competing technologies such as open cycle gas turbines. The demand response services considered variations in their response time and duration as well as commitment requirements. The financial viability of a demand response service committed continuously is shown to be strongly dependent on the utilisation of the pumps and the electricity tariffs used by water utilities. Through a range of water demand scenarios and financial incentives, we demonstrate how a WDS can participate in a demand response scheme and generate financial gains and environmental benefits.

Keywords: Demand Response, Water Distribution Systems, Pump scheduling, GHG emission mitigation, Optimisation

1. Introduction

Electricity storage schemes and grid management methods are becoming ever more important as the landscape of the electricity grid changes to more decentralised renewable production. The intermittent nature of these sources and the unavailability of contemporary technology for storing large quantities of electrical energy efficiently and cost effectively has led to a demand for new energy storage systems and more intelligent electricity demand management [52]. Edmunds et al. [14] highlight significant reductions in greenhouse gas (GHG) emissions from future UK power grids when storage technologies are implemented, while Lau et al. [23] show considerable GHG emission savings can be achieved through a range of demand response programs.

In demand response, an electricity consumer reduces its power consumption when requested to do so in exchange for compensation. For an electricity consumer with an electricity demand that is predictable into a future operational horizon, demand response (DR) is provided by reducing its electricity consumption compared to the predicted consumption [2]. Different DR mechanisms may impose requirements on how long the power reduction must last, how large it has to be, at what rate it must be reduced and within what timeframe it must be achieved. A detailed summary of possible mechanisms and their properties is provided by Ma et al. [26]. Despite the potential of active demand management to increase renewables penetration [16], a large share of demand response services is currently provided through backup generators instead of demand shifts by consumers [28].

Water utilities are a major electricity consumer, accounting for up to 5% of a city's electricity consumption [5]. Most of this energy is used to drive the pumps of the water distribution systems (WDS) [20]. The pumps are operated with schedules to guarantee adequate water supply for consumers. By making use of low tariff periods and the pumps' efficiency characteristics, the operating cost can be minimized through the optimisation of pumping schedules. With the emergence of smart power grids and the penetration of intermittent renewable power sources, existing operational paradigms are being questioned. The increased utilisation of renewable energies to power the operation of WDS has, for example, been shown to reduce the GHG emissions considerably [7]. We investigate how a WDS can provide reserve energy to a power grid through demand response. Three schemes available in the UK are used as case studies to evaluate the financial and environmental implications of the participation of WDS in such mechanisms.

^{*}Corresponding Author: email: ruben.menke08@imperial.ac.uk Tel.: +442075946018

The financial and environmental benefits from participating in a demand response scheme are assessed through a comparison of its optimised operational cost to that of normal operations that minimise only the operating cost for a given electricity tariff and water demand. To ensure this comparison is valid, we solve both schedules to a sufficiently small certifiable optimality gap; an optimality gap that is smaller than the model uncertainty is chosen [30]. When assessing the ability of a WDS to curtail its electricity usage at request to participate in the demand response market, we separate the hurdles to implementation into system and operational hurdles. The system constraints considered are the available financial rewards, the given electricity price structure and the water network's pump utilisation rate. These dictate whether a demand response program can be considered financially viable. Examples of operational constrains are ramp rates, pump switching constraints or minimum network pressure constraints. This investigation focusses on the system hurdles using quasi steady state modelling and simplified operating constraints; we assume the operational hurdles can be met with available control and monitoring technologies and design expertise.

This paper is organized as follows. First we describe the demand response mechanisms in section 2 and the assessment methods used for the financial and environmental benefits section 2.2. Based on our previous work on pump scheduling [30], we formulate the optimal scheduling problem for DR in section 3. The simulation of demand response events is described in section 4.1. Section 5 shows and discusses the results obtained from the a benchmark network. Finally, further work and conclusions are summarised in section 6.

2. Demand response

2.1. Service description



Figure 1: Approximate time scales for the National Grid response services to demand and supply mismatches. Adapted from National Grid [34]

In the United Kingdom, National Grid operates the electricity grid, maintaining it as tightly as possible around the desirable frequency of 50Hz. In case of a significant drop in frequency, as illustrated by Figure 1, National Grid recognises two mechanisms relevant for this work, frequency response and reserve energy. Within two seconds of an incident that causes the frequency to drop, the frequency response services are brought on-line to stabilise the grid. Reserve energy providers are then brought on-line within 20-30 minutes to enable the fast responding frequency services to be switched off to be used again at future events. The services considered here that can provide frequency response (FR) through demand response are the Firm Frequency Response (FFR) and Frequency Control by Demand Management (FCDM). The reserve energy provision service considered is the Short Term Operational Reserve (STOR).

The first demand response service considered for the WDS case studies is STOR since the technical requirements suggest that it can be implemented in a WDS more readily. A STOR provider offers a steady demand reduction and must deliver the reduction within 4 hours after being called and may be required to reduce the demand for up to 2 hours. However, the tender records show that the mean call duration in 2013 was 82 minutes and that National Grid prefers services that can respond within 10 - 20 minutes [28]. Since the minimum offered power requirement for STOR participation in the UK is 3MW, only large WDSs would be able to participate in a STOR scheme directly. However, through an aggregator, a company that aggregates several consumers and bids their capacities to National Grid, a smaller WDSs could participate in these mechanisms by sharing the profits generated with the aggregator. To offer STOR National Grid recognises a range of pathways to suit the wide range of suppliers [35]. The pathways modelled here are based on offering STOR services during both availability windows or just in one, this can be achieved through tendering either a committed or flexible service. The STOR windows and tariff structure is described in further detail in section 2.2 and in Figure 4.

The second method for demand response energy provision considered here is the provision of frequency response services through FFR or FCDM. National Grid requires that an FFR provider is able to deliver a minimum of 10MW response power; smaller users can offer FFR through an aggregator. For the secondary response service, which is considered here, the response must occur within seconds and be maintained for a few minutes. The service may be tendered for any time period, with National Grid preferring tenders that can offer and deliver the service most times. Furthermore, there are requirements detailing the metering and communication systems in place as well as pre-qualification assessments that need to be performed [40, 41]. FCDM is a bespoke service arranged through bilateral agreements with National Grid. In general an FCDM provider must provide the demand reduction within 2 seconds of instruction and deliver for a minimum of 30 minutes. The minimum demand reduction to be delivered is 3MW, which may be achieved by aggregating a number of smaller loads at same location. FCDM calls occur only ten to thirty times per annum [43].

For our analysis, the frequency response services FFR and FCDM are approximated by removing the minimum power delivery constraint and requiring the WDS to be able to deliver demand response throughout the day. The event duration for which water must be supplied to customers with reduced pump power is set to 30 minutes. For the analysis of the financial viability of DR the difficulties associated with sudden pump switches are neglected as we use quasi steady state models to represent the energy consumption; we consider the pump switching speed a technical issue to be resolved with the hydraulic (transient) modelling of the individual systems and their local control. A range of measures that can be employed to enable the pump switching speeds required for frequency response are discussed in Section 6.

Table 1: Summary of the range of monetary rewards for FFR and STOR service options [4, 42, 28, 41]. Rewards for FCDM are based on custom agreements with National Grid [43].

Туре	FFR	STOR	
Windows available:	24h	$1^{st} \& 2^{nd}$	1 <i>st</i>
Availability £/MW/h	80 - 150	7 - 9	1.8 - 2.5
Energy pay £/MWh	_	75 - 200	75 - 200
Total \pounds / MW (<i>R</i>)	45k-100k	30k - 45k	15k – 30k

Both FFR and STOR services are tendered at the beginning of the month or season respectively. Potential providers place bids with their cost and availability restrictions. Using historical data the range of energy provision and revenue generated from the reserve energy provision can be calculated. The financial rewards for a successful tender are summarised in Table 1. Frequency response services provide a small amount of energy - in a short (< 30*Minutes*) high power (> 10*MW*) burst - at short notice, while reserve energy services provide the reserve power over a longer time period but do not react as quickly as the frequency response services. This is reflected in the payment structure, where the revenue contribution of the availability payment is much more significant for frequency response services than for reserve energy services [28]. FFR and STOR are tendered in each of the six seasons National Grid considers and pricing varies significantly between seasons. The services are more lucrative for the providers in winter due to tighter margins in the grid [4]. The revenue achievable for STOR provision is based on 3436 to 3457 hours of allocated windows on workdays during which availability payments can be received and 90 minute mean duration of calls for STOR activity. Nonwork days are neglected in this analysis as they are less frequent and generally have a larger grid margin and fewer STOR events. For a more detailed summary of the effects and benefits of demand response in general applications, see Albadi and El-Saadany [2].

2.2. Environmental and Financial assessment

The environmental benefits of demand response from WDS is assessed by comparing the change in GHG emissions due to a change in operating schedule to the operational GHG emissions from competing energy storage mechanisms. The three largest power plants for STOR provision are open cycle gas turbines (OCGT), pumped hydro and diesel internal combustion engines [36, 39].

The GHG emissions attributed to the WDS operation are due to the time dependent energy consumption and the associated GHG emissions of the electricity grid. To define the GHG emissions caused by demand response the operations when providing demand response the operations when the service is available and when the response energy is provided are considered separately. The GHG emissions from either operation are compared to the normal operation when not providing demand response services. The cost of providing demand response are evaluated using the same method. However, as the GHG emissions are not explicitly minimised in the objective function the change in GHG emissions can be both negative or positive while the cost are minimised and thus a deviation from the normal optimal operating schedule is expected to incur a cost for the operation when providing demand response.

Table 2: Range of call frequencies, durations and resulting energy delivery in terms of committed power for the DR services considered [37, 41].

Туре	FFR	FCDM	STOR
Call frequency ~/annum	10-30	10-30	20-100
mean duration (min.)	30	30	82
Max. duration	30	_*	240
Energy provided (MWh/MW/a)) 5–15	15	27-137

*FCDM is based on bespoke agreements between National Grid and the supplier

The GHG emissions and cost due to the provision of response energy in the case of a demand response event can readily be calculated in terms of the energy provided during an event. To compute the GHG emissions and cost due to the availability to provide demand response, assumptions based about the usage of the service must be made as the GHG emissions and cost are incurred for the power provided and are independent of the actual energy delivered in events and are found from the difference to normal operations. These assumptions are summarised in Table 2. The analysis shows that the frequency response services FFR and FCDM deliver only a small amount of energy, due to the short nature of their responses, which is also reflected in the payment structure. The energy delivered from STOR in a given year varies by how often the provider is called, which is itself a function of the price of the the energy offered as National Grid uses a strict merit order system to call response energy [35].

3. Optimal Pump Scheduling

The optimisation of pump schedules for WDS operation is a difficult computational problem as the description of pump states and flow in pipes involve binary variables and some of the underlying fundamental system equations are non-linear. The integer problem can be solved through heuristic optimisation methods such as a genetic algorithm (GA) to optimise pump scheduling, with a separate solver for the non-linear hydraulic simulation problem [27, 54, 48, 50, 29]. Other heuristic procedures that have also been applied include simulated annealing [19, 45], ant colony optimisation [25] and particle swarm optimisation [55].

In the mathematical optimisation framework, the scheduling problem can be posed as an mixed integer problem (MIP), solving the hydraulic model and scheduling simultaneously. This can be solved using iterative linear programming for local optima [47] or using dynamic optimisation for small problems [53]. The scheduling problem can also be solved using branch and bound methods [18, 6]. To improve the solution speed, the hydraulic model can be simplified [3] or a Lagrangian relaxation [17] can be applied. A detailed review work on water distribution operation optimization is provided by D'Ambrosio et al. [13].

To compare different operational conditions and the resulting pump schedules, certifiable global optimality is required. This can be achieved through a branch and bound method [44]. A piecewise linear approach that approximates the problem such that it can be solved to global optimality is presented by Morsi et al. [32]. The optimal schedules are calculated using a piecewise linear approximation of the hydraulic constraints based on methods outlined in Menke et al. [30]. By comparing the operating cost and GHG emissions of a WDS participating in a demand response scheme to one under normal operations, we compute the optimal level of demand response capacity to provide, i.e. one that minimises the operating cost.

3.1. Optimization problem formulation

The optimisation problem for scheduling WDS pumps for DR can be formulated as:

minimise: Pump operation cost – DR revenue. *subject to*: Hydraulic constraints of pumps and pipes, mass balance of the system, additional constraints from DR provision.
(1)

In the following subsections, we describe the objective function and the physical, performance and DR constraints in more detail. In this section, we use nomenclature that refers to the network model in Figure 2 in order to explain WDS component modelling.

3.2. Pump operation cost

The decision variable in scheduling the operation of a fixed speed pump is the pump's state, ON or OFF, here described by $T_{i_p j} \in \{0, 1\}$ for pump i_p at time step $j \in [0, N]$. The energy consumption by the pumps in a WDS during a 24h period and the associated energy cost are calculated by a linear function:

$$f_1(\cdot) := \sum_{i_p=1}^{i_p=N_p} \sum_{j=1}^{j=N} T_{i_p,j} C_{i_p,j}$$
(2)

where $C_{i_p,j}$ is the cost of energy in having pump i_p ON at time *j*. Pump switching can have a negative effect on the maintenance cost of a system due to the changing loads contributing to transient or fatigue related failures. Penalising pump switching is often used to reduce this negative impact and account for maintenance cost [22, 50]. A penalty function that approximates the switching cost can be added to the objective function to lower the maintenance cost. When penalizing ON-to-OFF and OFFto-ON states equally it is given by:

$$f_2(\cdot) := \sum_{i_p=1}^{i_p=N_p} C_s \sum_{j=1}^{j=N} \left(T_{i_p,j} - T_{i_p,j-1} \right)^2$$
(3)

where C_s is the penalty for a single pump switch. The value of C_s is based on recommendations by Van Zyl et al. [54]. For further discussion of pump switching constraints and the cost associated with the dynamic response of a hydraulic system, see Section 6.

3.3. Hydraulic balance

The pressure delivered by a pump (i.e. the piezometric head difference across it) can be described by a set of linear constraints that define a convex set approximating the characteristic curve of a pump. For a given time step, a fixed speed pump i_p connecting nodes J1 and J2 is constrained by:

$$h_{J1} - h_{J2} \leq \begin{cases} m_{i_{p},1}^{p} q_{i_{p}} + c_{i_{p},1}^{p} T_{i_{p}} & \text{and} \\ m_{i_{p},2}^{p} q_{i_{p}} + c_{i_{p},2}^{p} T_{i_{p}} & \text{and} \\ \vdots & & \\ m_{i_{p},5}^{p} q_{i_{p}} + c_{i_{p},5}^{p} T_{i_{p}} & \text{if:} T_{i_{p}} = 1 \\ \Delta h_{ub}, \ q_{i_{p}} = 0 & \text{if:} T_{i_{p}} = 0 \end{cases}$$

$$(4)$$

where $m_{i_p,1} \dots m_{i_p,5}$ and $c_{i_p,1} \dots c_{i_p,5}$ are the linear coefficients for the five hyperplanes describing the convex set. Δh_{ub} is an upper bound on the pressure head generated by the pump. These constraints are enforced using a big-M method as detailed in Menke et al. [30] and Gleixner et al. [18].

Similarly, the energy balance for flows in pipes is modelled using a piecewise linear approximation of the head loss formulae given by either the Hazen-William or the flow dependent Darcy-Weisbach equations [1]. For a given time step, the head loss across pipe P2 connecting nodes J3 and J4, for example, can be approximated using a set of piece wise linear equations given by:

$$h_{J3} - h_{J4} = \begin{cases} q_{P2}m_{P2,1}^c + c_{P2,1}^c, & \text{if } q_{lim1} \le q_{P2} \le q_{lim2} \\ q_{P2}m_{P2,2}^c + c_{P2,2}^c, & \text{if } q_{lim2} \le q_{P2} \le q_{lim3} \\ \vdots \\ q_{P2}m_{P2,5}^c + c_{P2,5}^c, & \text{if } q_{lim5} \le q_{P2} \le q_{lim6} \end{cases}$$
(5)

where the five linear sections are given by $m_{P2,1}q_{P2} + c_{P2,1} \dots m_{P2,5}q_{P2} + c_{P2,5}$. Note that we have chosen to use five pieces after simulations showed that it was a sufficiently high order approximation for our purpose; see [30] for detailed analysis. In the optimization algorithm, these formulae are implemented using linear big-M constraints.

3.4. Mass balance at network nodes

Since steady-state approximations of the hydraulic conditions are used, compressibility effects are neglected and the mass flow is equal to the volume flow. For a network node joining components $P_1, P_2, \ldots P_n$, the mass flow must balance at each time step *j*. This is given by:

$$q_{P1,j} + q_{P2,j} + \dots + q_{Pn,j} = 0 \tag{6}$$

Demand at a node is considered in the mass balance and must always be met in feasible solutions. To ensure feasibility with respect to regulatory requirements to supply demand at sufficient pressures, a minimum hydraulic head constraints are enforced at demand nodes.

Tanks provide storage capacity in the networks providing water supply when the supply from the pumps is less than the demand. For a tank J with flows q_{in} and q_{out} the mass balance for time steps $j = 1 \dots N - 1$ is given by:

$$q_{in,j} + q_{out,j} = (h_{J,j+1} - h_{J,j}) \times A_J,$$
(7)

where the surface area of the tank is given by A_J . Since demand patterns are similar from day to day, we ensure that schedules are repeatable (reasonably similar) by enforcing the constraint that final levels in tanks do not differ much from their initial conditions:

where δ_V defines the volumetric difference. This relaxes the approach where the final or initial tank levels would be input data, which would limit the feasible search space and potentially lead to a sub-optimal final solution. A similar initial and final constraint relaxation is used in [47], while [11] includes a penalty for final water levels below the initial level or for ones away from a specified target at the end of the operating period.

3.5. Demand response cost function and constraints

The revenue from providing demand response is proportional to the power committed to the scheme. This is represented by:

$$f_3(\cdot) := -P_{dr} \times R \tag{9}$$

where P_{dr} is the power committed to DR and *R* is the expected revenue per MW committed as summarised in Table 1. However, to be able to provide this power as demand response, the WDS must certify that it would always consume this offered power unless it answers a DR call. The power consumed by the pumps is given by:

$$\sum_{j=1}^{j=n_p} P_{i,j} T_{i,j} \ge P_{dr} \tag{10}$$

When providing demand response, the WDS must be able to satisfy the expected water demand from consumers for the maximum possible duration of the DR event. This is enforced in the optimization model by specifying a minimum fill level for tanks, which depends on the maximum DR time offered. This increases the lower bound in the tank dependent on the demand level by:

$$h_{\min_{DR}J,i} = h_{\min_{norm}J,i} + \frac{V_{d\,J,i}}{A_J} \tag{11}$$

where $h_{min_{norm}J,i}$ is the lower bound of the tank level range and $V_{d J,i}$ is the volume deficit caused in tank J by operating the network without pumps for the maximum DR call duration at time step *i*. $V_{d J,i}$ is calculated using a hydraulic simulation before the optimisation and ensures the WDS can provide its water demand requirements during the demand response event.

The constraints for demand response provision are only enforced for time steps within the times for which demand response capability should be provided depending on the type of service being FFR where it is continuous or for STOR when it is during the first or both of the availability windows.

3.6. Problem summary

The variables of the optimisation problem are the binary variable T for the ON – OFF status of the pumps and the continuous variables h for the hydraulic head at a node and q for the volumetric flow rate in a connection. Although not described here, a set of binary indicator variables are used to enforce the piecewise linear approximations for headloss across pipes [30]. The power committed to demand response is given by P_{dr} . The pump schedule optimisation problem for a fixed electricity price tariff giving the normal operation schedule is given by:

min:
$$f_1(\cdot) + f_2(\cdot)$$

s.t.: (4), (5), (6), (7) (12)

For the demand response case where the provide level of demand response power is left to be found by the solver the above formulations are modified as:

min:
$$f_1(\cdot) + f_2(\cdot) + f_3(\cdot)$$

s.t.: (4), (5), (6), (7), (10), (11) (13)

The mixed integer quadratic program formulated in MAT-LAB and solved with CPLEX [9]. Since tighter bounds on variables can improve the computational speed of the branch and bound algorithm in CPLEX and we want to reduce the computational effort required to solve the problem to an adequate optimality, the bounds on the variables and the value of the big-M are chosen as tight as possible while including all hydraulically feasible solutions [30].



Figure 2: Van zyl network adapted from Van Zyl et al. [54]

To demonstrate demand response from water distribution systems, the Van Zyl benchmark network shown in Figure 2 is used as a case study. It is analysed under a range of pump utilisation rates, a range of overall rewards for the provision of DR and varying cost of energy through a selection of real electricity price tariffs. To enable these comparisons, the water demand is described in terms of the pump capacity of the network. In general, water demand at a node is modelled as the product of a time dependent term (the demand pattern) and a constant term (the base demand) [54]. The network model has



Figure 3: Water demand multiplication factor pattern at the demand node



Figure 4: Weekday electricity tariff of a UK water utility (top) and STOR windows for 2015 (bottom) - 17 [38]. Red highlights indicate the window times used here.

only one source reservoir and all water demand consumed in the network must flow through the pump station containing pumps main₁ and main₂. We express the pump utilisation of the network as a function of the best efficiency point (BEP) flow rate of one of these identical parallel pumps. To achieve this, the water demand was modified from the version available in Van Zyl et al. [54] by changing the base water demand (d_o) to the BEP flow rate of main₁ and modifying the pattern such that it had a mean of one. In simulating different levels of consumption compared to the capacity of each pump, the demand rate is modified, to a value d_s , and $\frac{d_s}{d_o}$ is defined as the pump utilisation rate. A low pump utilisation factor can be interpreted as a large pump supplying a network, while a higher factor indicates several smaller pumps supplying the network.

Larger commercial electricity consumers often utilise electricity tariffs with a range of prices across the day, with high peak prices during peak power consumption times. The electricity tariff used for this analysis is one used by a UK water utility and is shown in Figure 4. The peak price makes pump operations in this period particularly expensive. The changes in the electricity supply, due to the introduction of renewables, are expected to lead to a change in peak prices. However, the experiences and estimates from Australia and Germany [12, 46] show that the direction and scale of change in peak prices can vary significantly. To analyse the effect of peak price on the schedules and the financial viability of demand response services, we scale the tariff in Figure 4 by altering the minimum and maximum prices over a large range, while maintaining the same mean price for the tariffs to allow a fair comparison between different tariffs. Each electricity tariff used in our optimisation is thus referred to using the ratio of the maximum and minimum prices, $\frac{Price_{max}}{Price_{min}}$. The tariff used by the utility in Figure 4 has a ratio of 3.2.

Table 3: Cumulative probabilities of STOR DR event durations [37].						
Event (min.)	<30	30-60	60–90	90-120	120-150	>150
Share (%)	12.3	26.6	24.0	16.7	9.1	11.2

The time of the availability windows used for STOR services by National Grid vary from year to year and across seasons of the year; Figure 4 shows the STOR windows that are considered for this analysis highlighted in red, representing an aggregation of the window times offered by National Grid. In this analysis the availability window descriptions were simplified to the constraints that STOR service providers must be available to provide STOR services in both STOR windows or in just the first STOR window. A DR event can start only inside an availability window but may continue to outside of the window [35]; we model this explicitly. After participating in an event, there is a recovery period in which the STOR provider does not need to be available to provide another response. The likelihood of an event occurring at a specific time in the STOR window is considered uniform since the usage of historic statistics provided by National Grid [37] shows that there is no clear trend. The duration of STOR responses as well as their relative likelihood from historic STOR data are summarised in Table 3.

FFR services are provided throughout the day. However, a water utility can specify in its bid that it cannot provide the service at certain periods; this, however, reduces the benefit of the offer to National Grid. The requirement for FFR services in 2015 [41] shows a pattern with highest demand in the summer months and lowest in winter, while data of STOR services from 2014 shows no such clear pattern [37].

4. DR events

4.1. Simulating a DR event

The operation during and in the 24 hours after the begin of the event is investigated to provide an indication of the cost and GHG emissions that arise due to such an event. Compared to the original problem, which yielded \mathbf{T}^{o} , \mathbf{h}^{o} the original pump settings and tank levels, the event optimisation problem is further constrained, by the power consumption requirements of the event and the resulting changes in tank levels and deviation from the originally optimal path resulting from the optimal schedule.

Given the original operating schedule and the required power consumption reduction during an event the operating schedule during the event is computed a priori and together with the initial tank levels further constrain the optimisation problem. The initial and final tank levels areconstrained by:

$$h_{j,1} = h_{j,1}^o, \ h_{j,N} \ge h_{j,N}^o \quad \forall j \in \mathcal{J}_{Tanks}$$
(14)

where \mathcal{J}_{Tanks} is the set of tanks in the network.

The increased minimum tank levels as described by (11) cannot be enforced during the event and in the following recovery period. For a tank *J* during the event and the allowed recovery period the $\frac{V_{d,Ji}}{A_J}$ term is dropped and the minimum tank level is given by:

$$h_{\min_{DR}J,i} = h_{\min_{norm}J,i} \quad \forall i \in \mathcal{T}_{Event+recovery} \tag{15}$$

where $\mathcal{T}_{Event+recovery}$ is the time steps of the event and the recovery period after the event in which the WDS will not be asked to provide demand response again.

To verify how the tank levels and water provision of the WDS are affected by a DR event, these are modelled in a quasi steady state model. The optimal scheduling problem is formulated as:

$$\begin{array}{l} \min: f_1(\cdot) + f_2(\cdot) \\ \text{s.t.:} (4), (5), (6), (7), (10), (11), (14), (15) \end{array} \tag{16}$$

Further optimisation methods for a DR event are discussed in section 6.

4.2. Monte Carlo simulation of demand response events



Figure 5: GHG emissions for the UK grid for 2014 [15]. The emissions profile of each day is described by a trace with the mean highlighted in red. The emission intensity descriptions are discretised to 48 time steps as G.

The cost and GHG emissions from response energy provision depend on a range of factors. The electricity tariff used here is the same for all work days of the year, which are the only days considered for DR. The cost of a DR event, therefore, is dependent only on the duration and start time of the event and not the date. The range of daily GHG emission variations can influence the overall emissions [31]. To ensure this is captured in the estimate of the GHG emissions from events, the date of the simulated events is also varied and the corresponding historic GHG emission data used to compute the associated emissions. The traces of the date specific emission levels are shown in Figure 5.

The cumulative probabilities of STOR event durations are summarised in Table 3. For each 24-hour day, the modelled event durations are discretised into 30 minute intervals. The starting time of the events are modelled with a uniform probability within the STOR window and the probability of a STOR event occurring on a particular day is modelled as described in National Grid [37]. The simulation of FFR events was performed with similar considerations, however the event duration was always only one time step of 30 minutes. For a demand response event the operating schedule is computed following the procedure given below:

- 1. For a normal day operation (i.e. operation optimised for DR provision), solve (13) to get schedule T_0 , and record the initial fill levels of the tanks h_0 .
- 2. Solve the DR event simulation problem, which has additional constraints as described in (16).
- 3. The optimal schedule (\mathbf{T}_{Event}) for the operation with the event is computed
- 4. Compute the operation cost and GHG emissions of the event and compare to the original cost and GHG emissions



Figure 6: Variation of the normalised mean and standard deviation of cost and 00:00 GHG emissions of the Monte Carlo simulation for STOR events, showing their convergence well within 1000 simulated events

The number of events to model was chosen large enough to ensure convergence of the mean and standard deviation of the results. For example, Figure 6 shows the convergence of the standard deviation and mean cost and GHG emissions computed indicating that the analysis ha been performed on a sufficiently large sample [49]. These results of the simulations are are discussed in Section 5 and summarised in Table 4.

5. Results and Discussion

We investigate three aspects of demand response from WDS, how optimal pump operations change to enable the provision of demand response before and during a DR event, requirements needed for the provision of DR through pump scheduling to be financially viable and the environmental aspects of DR from WDS and how it compares to other alternative response energy provision technologies.

The results and discussion are separated into sections focussing on the financial viability of providing DR from WDS in Section 5.1, the GHG emissions associated with the provision of DR in Section 5.2, and the optimal scheduling for DR provision and DR events in Section 5.3, .



Figure 7: Reward and pump utilisation required for a given price pattern ratio for permanently committed demand response from FFR or FCDM.

5.1. Financial viability

Figure 7 shows the volume formed by the combinations of price ratios, reward size and pump utilisation rate that are financially viable to provide a FFR or FCDM service from the Van Zyl Network when assuming a maximum event duration of 30 minutes. It shows that a high pump utilisation rate, high reward and low price ratio benefit the financial viability of the fast response schemes. It also demonstrates that the ratio of the maximum and minimum prices of the electricity pattern as well as the pump utilisation rate have the strongest effect on the financial viability of the DR service. The reward level on the other hand has a lower impact on the financial viability. Demand response through FFR provides the highest amount of yearly revenue for committed power while requiring the least amount of energy provision. The revenue primarily stems from the availability payments for the power capacity provided and to a lesser extend from the energy provision. Assuming a WDS can fulfil the technical requirements with regards to size and pump switching speeds, it could provide a good opportunity for a profitable committed demand response provision if the pump utilisation rate and the electricity tariffs are moderately high. Otherwise, a bespoke FCDM agreement where the peak hours of the contract are spared out could provide a viable alternative.

The financial viability of STOR services is explored with an annual reward of £25000/MW, which represents the lower bound estimate of the revenue from availability payments alone, based on approximately 3500h of availability per year and £7-9/MW availability payments as detailed in Table 1. The optimal power level to commit to be available for STOR provision in both availability windows is shown in Figure 8. Figure 9 shows the same for provision in the first availability window only. The two figures show that for lower pump utilisation rates



Figure 8: Financially viable provision in both STOR windows for a reward of $\pounds 25000$ / a



Figure 9: Financially viable provision in only the first STOR window for a reward of ± 30000 / a.

no STOR service is viable as the optimum power is 0kW. For a small range of pump utilisation rates and electricity peak price ratios, STOR provision from the booster pump with 89kW is the optimal power for DR while for a large range of pump utilisation rates ≥ 1 a pump from the main pump station with 178kW can be committed to STOR provision. With a reward of £25000/MW, the additional revenue from DR provision, if viable, can be up to 4.6% of the normal operating cost.

The cost of scheduling for STOR relative to normal operations not only depends on the pump utilisation ratio and maximum price of the electricity tariff, but also on which availability windows STOR is offered in. If only the first STOR window is used, a wider range of conditions and a larger capacity of pump power can be committed to the provision of demand response. On the other hand, also providing STOR services in the second STOR window as well reduces the range of financially viable options as the peak electricity price becomes more relevant.



Figure 10: Distribution of the cost GHG emissions of energy provision from a Monte Carlo simulation of STOR DR events for $d_s/d_o = 1$.

Table 4: Summary of Monte Carlo simulation results, showing mean and 95th percentile of the cost and carbon intensity of the simulated energy provision results.

DR service	Cost(p/kWh)		$GHG (gCO_2 e/kWh)$	
type	\bar{x}	$Z_{0.05}$	\bar{x}	Z0.05
FFR	7.4	20.2	137	295
STOR	4.2	11.1	88	202

The cost of providing response energy in a STOR event are estimated by modelling a wide range of events in a Monte Carlo simulation, as shown in Figure 10. Table 4 shows that the cost of energy when rescheduling due to an event reduces the income generated through demand response event on average by 4.2 p/kWh and 95% of the events cost less than 11.1 p/kWh. This represents 25 – 50 % of the income generated from participating in the event. Thus, the cost associated with an event are shown to be well below the payments received for reserve energy provision.

For National Grid, STOR from a WDS would be very attractive since a water utility would not require a minimum guaranteed STOR provision to provide demand response; this is because, unlike for the competing technologies, STOR is only an additional revenue stream and not the sole purpose for a water utility [34].

5.2. Environmental analysis

The GHG emissions for the UK grid in 2014 are shown in Figure 5 and the carbon intensities of the competing technologies are summarised in Table 5. For the competing technologies providing STOR services, to be displaced by demand response from WDS, we consider the three most important technologies: Open Cycle Gas Turbines (OCGT), Pumped Hydro Storage

Table 5: Summary of carbon intensities of competing STOR provision technologies. Given in (gCO_{2e}/kWh)

Technology	best	worst
Open cycle gas turbine (OCGT)	480	575
Pumped hydro storage (PHS)	470	571
Internal Combustion Diesel (ICD)	520	700

(PHS) and Internal Combustion Diesel (ICD) engines, which together account for 82% of the STOR market [36]. When comparing the emissions of technologies, only the operational emissions are considered. For an OCGT plant these are provided by Seebregts et al. [51]. The PHS plant is assumed to fill its reservoirs in the early hours of the morning or at night, when average emissions are approximately 400 gCO_2 / kWh, this is based on data from [15]. The GHG emissions due to the consumption of electricity from pumped hydro storage is computed assuming an efficiency in the range of 70 - 85 % and neglecting life cycle related emissions, because the infrastructure is considered to be already in place [8]. The GHG emissions from operating an internal combustion diesel (ICD) engine and generator unit are computed by considering full load operation emissions only and neglecting emissions other than CO_2 [33]. The carbon intensity range used here is more generous towards the competing technologies than the figures given by National Grid [39], but as performance of power plants varies this range will give a better insight into the potential for GHG emission abatement through their displacement by DR from WDS.



Figure 11: Frequency and range of GHG emission levels for DR from STOR for the range of pump utilisation and price ratio investigated in Figure 8 and Figure 9. The carbon intensity is calculated assuming the maximum (i.e. 137 kWh/kW/a) from the range of energy provisions in Table 2.

The carbon intensity of providing the availability for demand response services by a WDS varies significantly. They can increase or decrease the overall emissions as they are not considered in the optimisation of the schedule and vary through out the day. However, the histogram in Figure 11 shows, the additional GHG emissions from scheduling for DR are contained to a range of $-50 - 50 \ gCO_2/kWh$.

The GHG emissions caused by the provision of response energy from the DR event are summarised in Figure 12. The mean GHG emissions as modelled in a Monte Carlo simula-



Figure 12: Distribution of GHG emissions of energy provision from a Monte Carlo simulation of STOR DR events for $d_s/d_o = 1$

tion are 88.3 CO_2/kWh and 95% of emissions are below 202.1 CO_2/kWh . Table 4 shows that the 95th percentile of the Carbon Intensity of the events are much less than the emissions from competing frequency and reserve energy providers summarised in Table 5. The total GHG emissions associated with the provision of response energy thus range from $-50 - 250 \ gCO_2/kWh$ with a mean of 90 and 95% of the energy provided with less than 240 gCO_2/kWh . This performance is significantly better than the next best conventional alternatives, an efficient OCGT or pumped hydro storage.

The network configurations yielding higher GHG emissions in Figure 5 are characterised by larger changes to operation schedule to facilitate a higher capacity of DR provision. However, there is no clear trend with regards to pump utilsation rate or electricity peak prices. Green house gas emissions from FFR are not considered here due to the small amount of energy displacement of the mechanism make the comparison to the values quoted in Table 5 misleading. However, it has been shown that frequency response services from PHS can reduce the GHG emissions compared to OCGT plants [23].

Generally the GHG emissions per unit of response energy provided by a WDS linearly depends on the total yearly response energy provided, in order to reduce the carbon intensity a lower price in the offer tender is suitable while higher prices may improve the profits at the expense of emission reductions.

5.3. Scheduling for demand response

Scheduling for DR divides into two problems: finding an optimal schedule to operate when providing the readiness to reduce energy usage at request and the scheduling during and after a DR event. The scheduling for the readiness requires the guaranteed operation and consumption of the energy tendered as demand response capacity. To schedule during and after a DR event, more operational constraints need to be considered; these include the initial tank fill levels, the minimum tank levels and the desired final tank levels.

To enable a meaningful comparison between schedules generated for a range of operating conditions the use of an optimisation method that can guarantee global optimality is necessary. However, the difficulty in solving the MIPs can lead to convergence issues with the solver not reaching the required level of optimality in the given maximum solve time. In the rare cases when the additional revenue from demand response is smaller than the optimality gap of the solution, this can lead to small variations of the results [30].



Figure 13: Simulated change in tank levels after a 90 minute DR event from 13:30AM onwards, showing the change in operating schedule and the resulting differences in tank levels. The tank levels are computed using a hydraulic simulation using a null-space algorithm [1].

Figure 13 exemplifies the development of the levels of the storage tanks and the change in the 24h operating schedules after a 90 minute DR call at 13:30PM on the 21st January 2014. The Figure highlights the resulting behaviour that occurs due to a DR call and the following deviation from the optimal pump schedule for normal operations without an event. The overall pump activity is increased with pumps operating in parallel more often. As a result, the overall energy consumption is increased.

Considering the minimisation of operating cost is only ever the second priority of a water utility, second to the guaranteed provision of water to the customers. This guarantee of supply needs to be also ensured in events and is verified through hydraulic simulations with the schedules. To ensure this feasibility, the approximation of the pump capacity underestimates the flow rate while the approximation of the head loss overestimates the head loss, ensuring the schedule provides sufficient energy to the system and the tank levels stay within the required bounds. For example, the capacity limit shown in red in Figure 7 is derived from hydraulic simulations; it shows that, in the marginal cases, the approximations lead to an underestimation of the pump capacity in the case of the upper end of the capacity limit.

6. Conclusion and future work

Through the use of a global optimisation technique we compared the operating schedules of a WDS system minimising the operating cost alone and minimising the operating cost while participating in different demand response schemes in the UK. Through this analysis we show that for a wide range of electricity tariffs and water demands there exist demand response mechanisms which allow the WDS to provide demand response and reduce its cost and provide response energy at low GHG emissions

STOR when tendering only for the first window can be provided at little extra cost to the WDS compared to regular optimal operation, as the highest price tariff times can be excluded from the provision period. Due to the specific requirements this poses, it may be necessary to provide STOR service through an aggregator or combine the STOR provision with another energy asset. When tendering to both windows the maximum price of the electricity tariff, if charged during the operation window, limits the financial viability of STOR provision.

The provision of response energy in an DR event is shown to have limited additional cost and GHG emissions. The operation scheduling during and after the event was performed using the same optimal scheduling techniques used to obtain the global optimal operating schedule. Through optimisation with a receding time horizon considering the uncertainty of future events occurring to allow repeated provision of demand response could further reduce the cost of providing DR through a better schedule.

The environmental impact is dependent on a range of factors, but demand response from WDS can often be provided at very low carbon intensity per unit of response energy provided. The faster responding services – FFR and FCDM – provide small amounts of energy potentially leading to worse carbon intensity of the energy provided, however the custom nature of FCDM may enable the inclusion of such services with small changes in scheduling and thus small changes in GHG emissions. Shorter response events have lower carbon intensities as the originally optimal schedule is only perturbed a little.

Electrical power distribution losses and life cycle emissions were ignored in this analysis since they are similar for the different technologies considered, for which we assume the infrastructure to be already in place. With the additional grid regulation services that come with the introduction of more renewables to the grid, the usage of already built WDS to regulate demand could provide significant reductions in GHG emissions compared to newly built infrastructure.

The WDS was modelled using quasi steady state modelling. To consider the delivery speed constraints of a network, more detailed modelling including the transient response of the network is necessary [10]. Further work could consider the different cost of surge protection devices to enable faster pump ramp rates without causing pressure induced failures in the pipes. Upgraded pump controls or battery systems to enable a gradual shut down may also be considered [24, 21].While the general notion of financial viability and GHG emissions from demand response from WDS should hold for other networks and topologies, the effect of the different aspects of the network topology are unclear and subject to further investigations.

Acknowledgements

Ruben Menke is funded by the Grantham Institute for Climate Change and the Environment, and Climate-KIC. Dr Edo Abraham is supported by the NEC-Imperial "Big Data Technologies for Smart Water Networks" project. Ruben Menke, Dr Edo Abraham and Dr Ivan Stoianov are members of the InfraSense Labs at Imperial College London.

References

- Abraham, E., Stoianov, I., 2015. Sparse null space algorithms for hydraulic analysis of large-scale water supply networks. Journal of Hydraulic Engineering 0 (0), 04015058.
- [2] Albadi, M., El-Saadany, E., Nov. 2008. A summary of demand response in electricity markets. Electric Power Systems Research 78 (11), 1989– 1996.
- [3] Biscos, C., Mulholland, M., Le Lann, M. V., Buckley, C. A., Brouckaert, C. J., 2003. Optimal operation of water distribution networks by predictive control using MINLP. Water SA 29 (4), 393–404.
- [4] Blair, N., Underhill, P., July 2015. TR26 Market Information Report. Tech. Rep. STOR-TR26-MIR, National Grid. URL http://www2.nationalgrid.com/UK/Services/ Balancing-services/Reserve-services/Short-Term-Operating-Reserve/ Short-Term-Operating-Reserve-Information/
- [5] Bunn, S., 2007. Closing the loop in water supply optimisation. In: Water Event, 2007 IET. IET, pp. 71–82.
- [6] Burgschweiger, J., Gnädig, B., Steinbach, M. C., Feb. 2008. Optimization models for operative planning in drinking water networks. Optimization and Engineering 10 (1), 43–73.
- [7] Campana, P. E., Li, H., Yan, J., dec 2013. Dynamic modelling of a PV pumping system with special consideration on water demand. Applied Energy 112, 635–645.
- [8] Chen, H., Cong, T. N., Yang, W., Tan, C., Li, Y., Ding, Y., Mar. 2009. Progress in electrical energy storage system: A critical review. Progress in Natural Science 19 (3), 291–312.
- [9] CPLEX, IBM ILOG, V12. 1: Users manual for cplex, International Business Machines Corporation 46 (53) (2009) 157.
- [10] Collins, R. P., Boxall, J. B., Karney, B. W., Brunone, B., Meniconi, S., Apr. 2012. How severe can transients be after a sudden depressurization? Journal - American Water Works Association 104 (1), E243–E251.
- [11] Crawley, P. D., Dandy, G. C., Jan. 1993. Optimal Operation of MultipleReservoir System. Journal of Water Resources Planning and Management 119 (1), 1–17.
- [12] Cutler, N. J., Boerema, N. D., MacGill, I. F., Outhred, H. R., oct 2011. High penetration wind generation impacts on spot prices in the Australian national electricity market. Energy Policy 39 (10), 5939–5949.
- [13] D'Ambrosio, C., Lodi, A., Wiese, S., Bragalli, C., June 2015. Mathematical Programming techniques in Water Network Optimization. European Journal of Operational Researc.
- [14] Edmunds, R., Cockerill, T., Foxon, T., Ingham, D., Pourkashanian, M., Jun. 2014. Technical benefits of energy storage and electricity interconnections in future British power systems. Energy 70, 577–587.
- [15] Elexon, 2015. Balancing reports. URL http://www.bmreports.com/

- [16] Fürstenwerth, D., Waldmann, L., 2014. Stromspeicher in der Energiewende. Tech. rep., Agora Energiewende, Berlin. URL http://www.agora-energiewende.org/topics/ optimisation-of-the-overall-system/detail-view/article/ studie-die-energiewende-muss-nicht-auf-stromspeicher-warten/
- [17] Ghaddar, B., Naoum-Sawaya, J., Kishimoto, A., Taheri, N., Eck, B., Mar. 2015. A Lagrangian decomposition approach for the pump scheduling problem in water networks. European Journal of Operational Research 241 (2), 490–501.
- [18] Gleixner, A., Held, H., Huang, W., Vigerske, S., Nov. 2012. Towards globally optimal operation of water supply networks. Numerical Algebra, Control and Optimization 2 (4), 695–711.
- [19] Goldman, F. E., Mays, L. W., 1999. The application of simulated annealing to the optimal operation of water systems. In: 29th Annual Water Resources Planning and Management Conference.
- [20] Grundfos Managment A/S, 2004. Pump Handbook. Grundfos Managment A/S, Bjerringbro, Denmark.
- [21] Lambert, A., 2000. What do we know about pressure-leakage relationships in distribution systems. In: IWA Conf. n Systems approach to leakage control and water distribution system management.
- [22] Lansey, K. E., Awumah, K., 1994. Optimal pump operations considering pump switches. Journal of Water Resources Planning and Management 120 (1), 17–35.
- [23] Lau, E., Yang, Q., Stokes, L., Taylor, G., Forbes, A., Clarkson, P., Wright, P., Livina, V., dec 2015. Carbon savings in the UK demand side response programmes. Applied Energy 159, 478–489.
- [24] Lingireddy, S., Wood, J. D., Dec. 1998. Improved Operation of Water Distribution Systems Using Variable-Speed Pumps. Journal of Energy Engineering 124 (3), 90–103.
- [25] López-Ibáñez, M., Prasad, T. D., Paechter, B., Jul. 2008. Ant Colony Optimization for Optimal Control of Pumps in Water Distribution Networks. Journal of water resources planning and management 134 (4), 337–346.
- [26] Ma, O., Alkadi, N., Cappers, P., Denholm, P., Dudley, J., Goli, S., Hummon, M., Kiliccote, S., MacDonald, J., Matson, N., Olsen, D., Rose, C., Sohn, M. D., Starke, M., Kirby, B., O'Malley, M., Dec. 2013. Demand Response for Ancillary Services. IEEE Transactions on Smart Grid 4 (4), 1988–1995.
- [27] Mackle, G., 1995. Application of genetic algorithms to pump scheduling for water supply. In: 1st International Conference on Genetic Algorithms in Engineering Systems: Innovations and Applications (GALESIA). Vol. 1995. IET, IEE, pp. 400–405.
- [28] Macleod, L., 2012. Overview of National Grids Balancing Services. Tech. rep., Ofgem.

URL https://www.ofgem.gov.uk/ofgem-publications/56997/ national-grid-presentation-demand-side-response-event-autumn-2012. pdf

- [29] Marques, J. a., Cunha, M., Savić, D. A., Jan. 2015. Multi-objective optimization of water distribution systems based on a real options approach. Environmental Modelling & Software 63 (0), 1–13.
- [30] Menke, R., Abraham, E., Stoianov, I., Parpas, P., 2015. Approximation of system components for pump scheduling optimisation. In: 13th Computer Control for Water Industry Conference, CCWI 2015.
- [31] Messagie, M., Mertens, J., Oliveira, L., Rangaraju, S., Sanfelix, J., Coosemans, T., Van Mierlo, J., Macharis, C., dec 2014. The hourly life cycle carbon footprint of electricity generation in Belgium, bringing a temporal resolution in life cycle assessment. Applied Energy 134, 469–476.
- [32] Morsi, A., Geißler, B., Martin, A., 2012. Mixed Integer Optimization of Water Supply Networks. In: Mathematical Optimization of Water Networks. Springer Basel, Basel, Ch. 3, pp. 35–54.
- [33] MWM, 2011. TCD 2020. Tech. rep. Version 11/08/E, MWM. URL http://www.deutz.co.za/pdf_documents/mwm/ brochure-tcd2020-diesel.pdf
- [34] National Grid, 2010. Short Term Operating Reserve Tender Assessment Principles. Tech. Rep. 3, National Grid.
- [35] National Grid, 2015. General Description of the Service. Report. SCT Issue 9
- [36] National Grid, 2013. Short Term Operating Reserve Fuel Type Analysis -Season 7.3. Tech. rep. (19/08/13) 23/09/13)
- [37] National Grid, 2014. STOR Market Report 2013-14. Report, STOR REVIEW 2013 - 2014 National Grid.

URL http://www2.nationalgrid.com/WorkArea/DownloadAsset.aspx?

id=40521

- [38] National Grid, 2015. 2015 STOR Tender Round Calendar. Tech. rep., National Grid.
- [39] National Grid, 2015. Carbon Intensity Report. Tech. rep., National Grid.[40] National Grid, 2015. Firm Frequency Response (FFR).
- URL http://www2.nationalgrid.com/uk/services/balancing-services/ frequency-response/firm-frequency-response/
- [41] National Grid, 2015. Firm Frequency Response Review. Tech. Rep. May, National Grid.

URL http://www2.nationalgrid.com/WorkArea/DownloadAsset.aspx? id=40837

- [42] National Grid, 2015. Post Assessment Tender Report July 2015. Tech. rep., National Grid. URL http://www2.nationalgrid.com/UK/Industry-information/ Electricity-transmission-operational-data/Report-explorer/ Services-Reports/
- [43] National Grid, Sims, A., 2015. Frequency Control by Demand Managment.

URL http://www2.nationalgrid.com/uk/services/balancing-services/ frequency-response/frequency-control-by-demand-management/

- [44] Nemhauser, G. L., 1966. Introduction to dynamic programming. John Wiley & Sons.
- [45] Pedamallu, C. S., Ozdamar, L., 2008. Investigating a hybrid simulated annealing and local search algorithm for constrained optimization. European Journal of Operational Research 185 (3), 1230–1245.
- [46] PowerSolution Energieberatung, 2015. Energiemarkt Info 03—04 2015. Tech. rep., Vienna.
- [47] Price, E., Ostfeld, A., 2013. Iterative Linearization Scheme for Convex Nonlinear Equations: Application to Optimal Operation of Water Distribution Systems. Journal of Water Resources Planning and Management 139 (3), 299–312.
- [48] Reis, L. F. R., Bessler, F. T., Walters, G. A., Savic, D., Apr. 2006. Water Supply Reservoir Operation by Combined Genetic Algorithm Linear Programming (GA-LP) Approach. Water Resources Management 20 (2), 227–255.
- [49] Robert, C., Casella, G., 2010. Introducing Monte Carlo Methods with R. Springer New York, New York, NY.
- [50] Savic, D. A., Walters, G. A., Schwab, M., 1997. Multiobjective genetic algorithms for pump scheduling in water supply. In: Evolutionary Computing. Springer, pp. 227–235.
- [51] Seebregts, A. J., Simbolotti, G., Tosato, G., 2010. Gas-Fired Power. Tech. Rep. April, IEA. URL http://www.iea-etsap.org/web/E-TechDS/PDF/E02-gas_fired_power-GS-AD-gct.pdf
- [52] Strbac, G., Dec. 2008. Demand side management: Benefits and challenges. Energy Policy 36 (12), 4419–4426.
- [53] Ulanicki, B., Kahler, J., See, H., Jan. 2007. Dynamic Optimization Approach for Solving an Optimal Scheduling Problem in Water Distribution Systems. Journal of Water Resources Planning and Management 133 (1), 23–32.
- [54] Van Zyl, J. E., Savic, D. A., Walters, G. A., 2004. Operational optimization of water distribution systems using a hybrid genetic algorithm. Journal of Water Resources Planning and Management 130 (2), 160–170.
- [55] Wegley, C., Eusuff, M., Lansey, K., 2000. Determining Pump Operations using Particle Swarm Optimization. In: Hotchkiss, R. H., Glade, M. (Eds.), Building Partnerships. American Society of Civil Engineers, pp. 1–6.